

Characteristics of the Impact Pressure of Debris Flows

Hongchao Zheng¹ , Zhenming Shi¹, Tjalling de Haas² , Danyi Shen¹ , Kevin J. Hanley³, and Bo Li¹ 

¹Department of Geotechnical Engineering, College of Civil Engineering, Tongji University, Shanghai, China, ²Department of Physical Geography, Utrecht University, Utrecht, The Netherlands, ³School of Engineering, Institute for Infrastructure and Environment, The University of Edinburgh, Edinburgh, UK

Key Points:

- Stationary mean pressure (SMP) and fluctuating pressure are obtained from the measured impact pressure signals
- Peak SMP are predicted well with a jet model based on the Bernoulli equation
- The transition in the dependence of grain segregation on viscosity coincides with the transition of the flow rheology

Supporting Information:

Supporting Information may be found in the online version of this article.

Correspondence to:

D. Shen,
1107sdy@tongji.edu.cn

Citation:

Zheng, H., Shi, Z., de Haas, T., Shen, D., Hanley, K. J., & Li, B. (2022). Characteristics of the impact pressure of debris flows. *Journal of Geophysical Research: Earth Surface*, 127, e2021JF006488. <https://doi.org/10.1029/2021JF006488>

Received 18 OCT 2021
Accepted 3 MAR 2022

Abstract Debris flows are common geological hazards in mountainous regions worldwide. Predicting the impact pressure of debris flows is of major importance for hazard mitigation. Here, we experimentally investigate the impact characteristics of debris flows by varying the concentrations of debris grains and slurry. The measured impact pressure signal is decomposed into a stationary mean pressure (SMP) and a fluctuating pressure (FP) through empirical mode decomposition. The SMP of low frequency is caused by the thrusting of bulk flow while the FP of high frequency is induced by the collision of coarse debris grains, revealed by comparing the features of impact pressure spectra of pure slurries and debris flows. The peak SMP and the peak FP first increase and then decrease with the slurry density. The basal frictional resistance is reduced by the nonequilibrium pore-fluid pressure for debris flows with low-density slurry, which can increase the flow velocity and impact pressures. In contrast, the viscous flow of high-density slurry tends to reduce the flow velocity. The peak SMPs are well predicted by the Bernoulli equation and are related to the hydrostatic pressure and Froude number of the incident flow. The peak FPs depend on the kinetic energy and degree of segregation of coarse grains. The maximum degree of segregation occurs at an intermediate value of slurry density due to the transition of flow regime and fluid drag stresses. Our results facilitate predicting the impact pressures of debris flows based on their physical properties.

Plain Language Summary Debris flows are mixtures of muddy water, sand, gravels, and boulders which move down steep mountain creeks in an uncontrolled way. They are a major threat to human life, properties, and infrastructure in mountainous regions. Debris flows commonly consist of a flow nose made of coarse-grained particles and a flow body comprising finer-grained and more liquefied debris. It is very important to predict their impact pressures which are significantly influenced by their flow behavior. In this study, the measured impact pressures of experimental debris flows were decomposed into several components through a signal processing method. The low-frequency components of the signal originated from the bulk flow and the high-frequency components were caused by the coarse debris grains. This decomposition inspired us to predict separately the pressures induced by bulk flow and coarse debris grains to obtain the peak impact pressure of a debris flow.

1. Introduction

Debris flows are gravity-driven mass movements in mountainous regions (Iverson, 1997; Pudasaini, 2012). Peak flow velocities of debris flows can surpass 10 m/s, and volumes can reach $\sim 10^9$ m³ (e.g., Iverson et al., 2011). Debris flows can cover floodplains, block rivers and deteriorate the regional ecological environment (Takahashi, 2007; Zheng et al., 2018, 2021a). They further pose a major threat to human life, properties, and infrastructure (de Haas et al., 2015; Kaitna et al., 2016; Zheng, Shi, Hanley, et al., 2021).

A fundamental problem in disaster prevention engineering is to determine the impact pressure exerted on structures by a debris flow (Sovilla et al., 2008). This task is difficult because the impact pressure of a debris flow depends on both solid and fluid stresses that influence their motion and govern their rheological properties (de Haas et al., 2015, 2021). Debris flows typically have a wide grain-size distribution including sediment particles ranging in size from clay to boulders (de Haas et al., 2021; Iverson, 1997). The heterogeneous grain distribution of a debris flow in the longitudinal direction can also affect their impact pressures (McCoy et al., 2010).

Particle size segregation is a common feature of debris flows: coarse particles tend to migrate towards the front of the flow and fines toward the rear (Kaitna et al., 2016; Zheng, Shi, Yu, et al., 2021). Flow behavior is altered

in time and space, often causing a flow snout with high frictional resistance, flow fingering and levee formation (de Haas et al., 2015; Gray & Kokelaar, 2010; Johnson et al., 2012; Pudasaini & Fischer, 2020; Vallance & Savage, 2000). The impact pressure of a debris flow can be amplified as a result of a large concentration of coarse grains at the flow front (Hu et al., 2011; Hungr et al., 1984; Watanabe & Ikeya, 1981). However, the relation between grain segregation and the impact pressure is still unclear.

Current debris-flow impact models can be classified into hydraulic and solid-collision models (Hubl et al., 2009). This twofold classification indicates the complexity of debris-flow processes, where the impact can either be caused by fluid-phase flow thrusting or a point-wise loading and the collision of coarse grains (Scheidl et al., 2013). The hydraulic models are further classified into hydrostatic and hydrodynamic models. In general, the hydrostatic model is expressed as

$$p_{\text{peak}} = k\rho_bgh \quad (1)$$

Here, p_{peak} is the peak debris-flow impact pressure with the parameter k as an empirical factor; ρ_b and h are the density and depth of a debris flow. The hydrodynamic model based on the impulse–momentum theorem is expressed as

$$p_{\text{peak}} = \lambda\rho_bv^2 \quad (2)$$

with the impact coefficient λ and the flow velocity v . For the former, the peak impact pressure measured from hillslope debris flows is typically 2–50 times the equivalent static flow pressures (Bugnion et al., 2012). For the latter, the back-calculated λ for the hydrodynamic models ranges from 0.4 to 17.0 (Scheidl et al., 2013). These fitted parameters with a wide range bring great uncertainty to designs in disaster prevention engineering.

Debris flows generally contain 40%–70% sediment particles by volume (de Haas et al., 2015; Pierson, 2005). The volume concentration of debris flows may significantly affect the impact pressure based on the hydraulic and solid-collision models. The hydrostatic pressures exhibit a linear relationship with the volume concentration of debris flows. Meanwhile, the interstitial slurry of a debris flow can be highly viscous because of the suspension of clay and silt particles in the interstitial water (e.g., Costa, 1988). Viscous slurry may facilitate nonequilibrium pore-fluid pressures in the flow, thereby enhancing flow velocity and impact pressures by decreasing the inter-granular friction (Hsu et al., 2014; Iverson, 2003) and dampening grain collisions (Kaitna et al., 2016; Vallance & Savage, 2000). Further research is therefore needed to investigate the independent effects of volume concentration of debris grains and ambient slurry on the debris-flow dynamics and impact pressures.

The measured impact pressure signals of debris flows are characterized by a stationary mean value superimposed by fluctuations (Bugnion et al., 2012; P. Cui et al., 2015; Hu et al., 2011; Hubl et al., 2009; Scheidl et al., 2013; Sovilla et al., 2008). The local fluctuations can result from environmental noise, resonance frequency of measuring apparatus, point-wise loading or hard impact of coarse grains, etc. (Bugnion et al., 2012; Scheidl et al., 2013). Filtering or moving average methods are usually adopted to eliminate the fluctuations of impact pressure, considering that the durations of oscillation are very short (Bugnion et al., 2012; Scheidl et al., 2013). The peak impact pressures of debris flows are thus underestimated as a result of such filtering procedures. Instead, a comprehensive decomposition of fluctuation signals based on their specific sources can efficiently improve the prediction of actual impact pressures.

To fill the knowledge gaps regarding impact pressure characterization, we conduct impact experiments of debris flows with varying concentrations of debris grains and slurry. One goal of this study is to be able to predict impact pressures from fundamental flow properties. The measured impact pressure signal from each test is decomposed into a stationary mean pressure (SMP) and local fluctuations from different sources through empirical mode decomposition (EMD) (Huang et al., 1998). Subsequently, we establish a jet model (Song et al., 2021) to predict the SMP. The degree of segregation of coarse grains is assessed by the impulse–momentum theorem (Bugnion et al., 2012). Finally, we discuss the fluctuating pressure (FP), impact coefficient of debris flow and segregation of coarse grains associated with the transition of flow regime.

Table 1
Parameters for the Different Tests

| Test | ρ_b (kg/m ³) | ρ_s (kg/m ³) | ρ_d (kg/m ³) | ρ_c (kg/m ³) | C_v | C_d |
|------|-------------------------------|-------------------------------|-------------------------------|-------------------------------|-------|-------|
| 1 | 1000 | 1000 | 0 | 0 | 0.00 | 0 |
| 2 | 1100 | 1100 | 0 | 0 | 0.06 | 0 |
| 3 | 1200 | 1200 | 0 | 0 | 0.12 | 0 |
| 4 | 1300 | 1300 | 0 | 0 | 0.18 | 0 |
| 5 | 1350 | 1350 | 0 | 0 | 0.21 | 0 |
| 6 | 1680 | 1000 | 1080 | 227 | 0.40 | 0.40 |
| 7 | 1740 | 1100 | 1080 | 227 | 0.44 | 0.40 |
| 8 | 1800 | 1200 | 1080 | 227 | 0.47 | 0.40 |
| 9 | 1860 | 1300 | 1080 | 227 | 0.51 | 0.40 |
| 10 | 1890 | 1350 | 1080 | 227 | 0.52 | 0.40 |
| 11 | 1748 | 1000 | 1188 | 250 | 0.44 | 0.44 |
| 12 | 1804 | 1100 | 1188 | 250 | 0.47 | 0.44 |
| 13 | 1860 | 1200 | 1188 | 250 | 0.51 | 0.44 |
| 14 | 1916 | 1300 | 1188 | 250 | 0.54 | 0.44 |
| 15 | 1944 | 1350 | 1188 | 250 | 0.56 | 0.44 |
| 16 | 1816 | 1000 | 1296 | 272 | 0.48 | 0.48 |
| 17 | 1868 | 1100 | 1296 | 272 | 0.51 | 0.48 |
| 18 | 1920 | 1200 | 1296 | 272 | 0.54 | 0.48 |
| 19 | 1972 | 1300 | 1296 | 272 | 0.57 | 0.48 |
| 20 | 1998 | 1350 | 1296 | 272 | 0.59 | 0.48 |

Note. ρ_s and ρ_b are the densities of slurry and bulk flow, respectively. ρ_d and ρ_c are the dry densities of debris grains and coarse grains (5–8 mm), respectively. $\rho_d = \rho_g C_d$ where ρ_g is the solid particle density (2,700 kg/m³) and C_d is the volume concentration of debris grains. C_v is the volume concentration of debris flow. $\rho_b = \rho_g C_v + \rho_w (1 - C_v)$, where ρ_w is the water density (1,000 kg/m³).

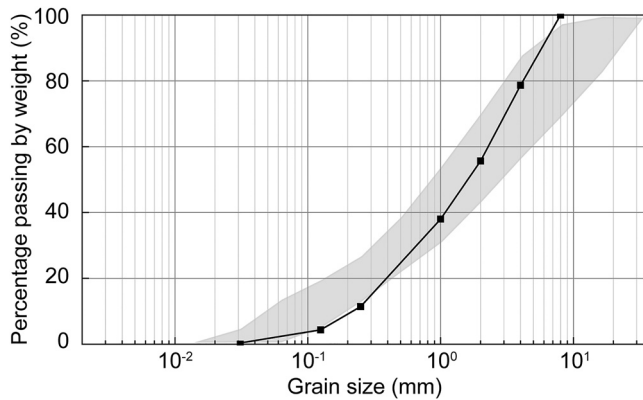


Figure 1. Compositions of debris grains (black line) in our tests and debris flows of the “Inferno” type in the Susa Valley of the Italian Alps (gray zone) (Tiranti et al., 2008).

2. Methods

2.1. Debris-Flow Materials

We conducted 15 impact tests of debris flows with varying debris-grain concentration C_d ranging from 0.40 to 0.48 and the slurry density ρ_s ranging from 1,000 to 1,350 kg/m³ (Table 1). The debris grain composition (0.125–8 mm) of the experimental flows is similar to the composition of the “Inferno” type debris flows in the western Italian Alps, as shown in Figure 1 (Tiranti et al., 2008). The slurry in these flows was a mixture of water and hydrous kaolin (0.001–0.01 mm). Five additional tests (tests 1–5) were conducted with pure slurries, that is, without debris grains, to compare the impact pressures with those of flows with the same slurry density.

Slurry rheology was measured by a concentric cylinder viscometer (Anton Paar, MCR 301). The shear rate was 0.1–100 s⁻¹ at a temperature of 20°C. The dynamic viscosity η of slurry increased significantly when the slurry density exceeded 1,200 kg/m³ (Figure 2). We conducted direct shear tests (Humboldt, D-5780) to obtain the internal friction angle ϕ under a drainage condition, where the normal stresses were 50, 100, 150, and 200 kPa, the shear rate was 1 mm/min, and the shear displacement was 60 mm.

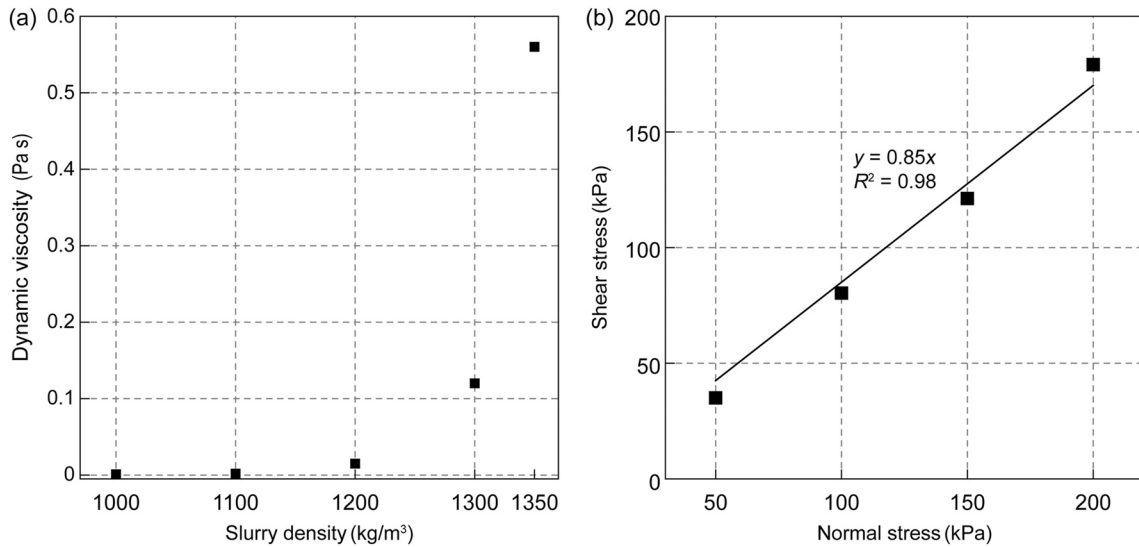


Figure 2. (a) Dynamic viscosity of slurry and (b) internal friction angle of debris. R^2 is the coefficient of determination for the linear regression in panel b.

2.2. Flume Setup

The experimental apparatus consisted of a mixing tank, a lever system and a straight-slope flume (Figure 3 and Figure S1 in Supporting Information S1). The mixing tank with a volume of 0.07 m³ was used to store and mix the flow. A 0.5-m high vertical headgate was equipped to retain debris flow prior to its release. The headgate was constrained by a steel rod with a length of ~2.0 m, which was a part of the lever system. A rubber seal inside the headgate ensured that slurry in the mixing tank would not leak out.

The experimental flume had a height of 0.4 m, a width of 0.25 m, a length of 4.0 m, and an angle θ of 27° to the horizontal plane. The flume sidewalls were made of transparent tempered glass, allowing the impact process of released flow to be observed. The bottom of the flume was pattern steel roughened by small bulges with a roughness height of 1.6 mm, matching the median diameter d_s of the debris flows to simulate natural channel roughness.

The instruments used for investigating the impact process of debris flows are shown in Figure 3. A high-speed camera (i-SPEED7, iX Cameras) with a sample frequency of 200 Hz captured the flow characteristics in the cross-stream direction. The flow-front height h normal to the flume bottom and velocity v were obtained from the snapshots and scale plate. Three video cameras (GZ-R10BAC, JVC, 1,920 × 1,080 pixel) recorded the movement process of debris flows from the top of the flume. At $x = 3.2$ m we deployed pore pressure and stress sensors to

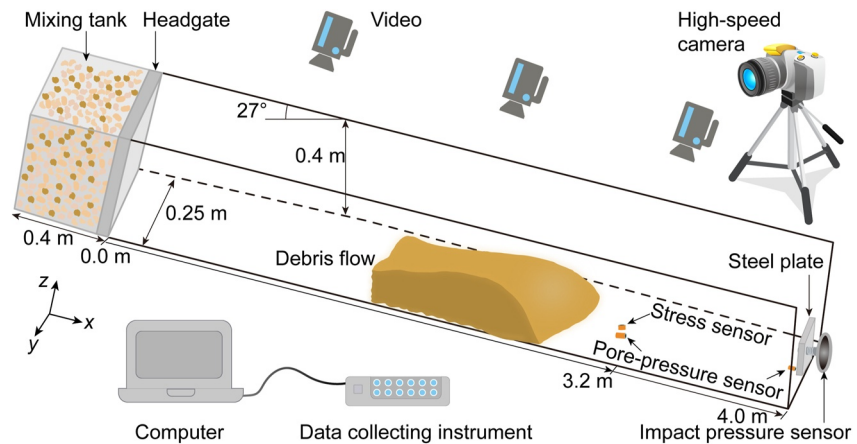


Figure 3. Schematic diagram of experimental apparatus.

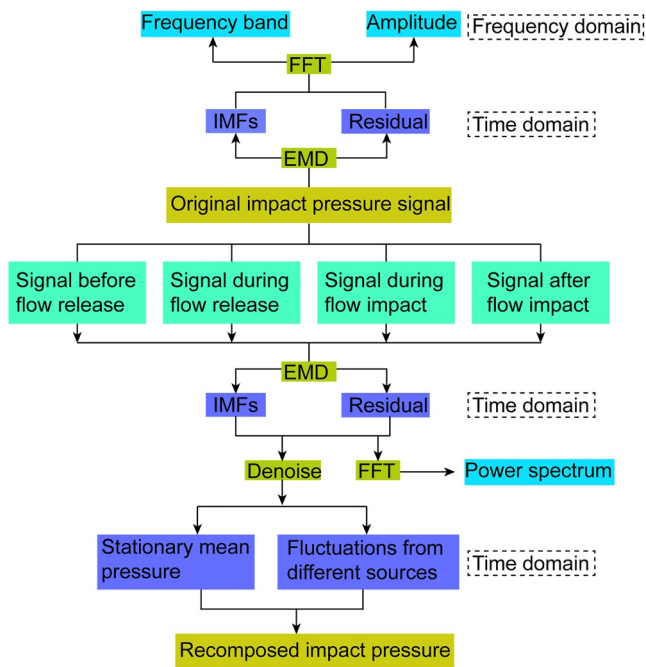


Figure 4. Analysis procedure for the impact pressure signals. The signal before flow release is used to observe the noise in the system.

measure the basal pore pressure and basal normal stress during debris flow propagation. At $x = 4.0$ m we deployed a pore-pressure sensor to measure the local dynamic fluid pressure during the impact process. The pore-pressure sensors were saturated and accommodated in a cavity on the flume bottom. Calibrations of pore pressure and stress sensors using static water pressures yielded regression line slopes that were both linear (determination coefficient $R^2 > 0.99$) and reproducible. A square steel panel (80×80 mm) was mounted to the strain-gauge sensor (Baumer, DLRP, range ± 200 N) to bear the impact pressure of debris flows at the flume exit (Figure S2 in Supporting Information S1). A rigid block was welded to the bottom of the flume to retain the strain-gauge sensor. The sample frequency of all electronic sensors was 2,000 Hz.

2.3. Experimental Procedure

The volume of released flow V_0 for each test was 0.055 m^3 . The debris was prepared as follows. First, the masses of debris grains and slurry in each flow were calculated according to the required concentration. Then, the debris material was poured into the mixing tank along with water injection. Finally, the debris was stirred by two blenders prior to and during release to ensure that coarse grains were well mixed with the slurry rather than depositing at the bottom of the mixing tank.

We initiated flow using the lever system to release the restraint on the side-hinged headgate. This release, combined with the static force of debris flows bearing on the headgate and counterforce on the lever, caused it to swing

open horizontally in about 0.5 s. The resulting flows began as nearly ideal dam-break flows and gradually accelerated during downstream propagation.

2.4. Decomposition of Impact Pressure Signals

The impact pressure signal of a debris flow is typically non-stationary and non-linear, and normally contains local fluctuations arising from different sources (e.g., P. Cui et al., 2015; Hubl et al., 2009). With EMD, this complicated signal can be adaptively decomposed into a sum of a finite number of zero mean oscillating components termed intrinsic mode functions (IMFs) without a priori basis function selection (Huang et al., 1998). EMD is based on the sequential extraction of energy with intrinsic time scales of the signal from high to low frequencies (Maheshwari & Kumar, 2014). A physically meaningful characterization of the signal can be obtained. Here, the EMD method is employed to decompose each impact pressure signal into a SMP with a lower frequency and local fluctuations with higher frequencies (Figure 4). We first decomposed the impact pressure signal p_0 during the entire sampling process into IMFs and a residual r sequentially from high to low frequencies:

$$\begin{aligned}
 p_0(t) &= \text{IMF}_1(t) + \text{IMF}_2(t) + \dots + \text{IMF}_n(t) + r(t) \\
 &= \sum_{i=1}^n \text{IMF}_i(t) + r(t)
 \end{aligned}
 \tag{3}$$

where IMF_i is the i th IMF and n is the total number of IMF components. A detailed description of the EMD decomposition process is provided in Supporting Information S1 document. The overall frequency spectra of the IMFs and the residual including their frequency band and amplitude during the entire process were obtained by the fast Fourier transform (FFT). An IMF is defined as a function that meets two conditions: (a) the number of zero-crossings and extremes must either be the same or differ at most by one, and (b) the envelopes defined by the local maxima and minima should be symmetric (Huang et al., 1998). The residual r does not need to satisfy the requirements of an IMF and takes the form of a function or a constant value. Then, impact pressure signals at different stages, delineated from a signal curve (before flow release, during flow release, during flow impact, and after flow impact), are decomposed with EMD. The respective frequency spectra of the IMFs and the residual at

different stages were obtained. The signal sources of the IMFs with different frequencies were distinguished by comparing frequency spectra in these four stages.

EMD and FFT are invoked twice during the processing of impact pressure. On the first occasion, the overall frequency spectra of the IMFs and residual during the test are obtained. On the second occasion, the frequency spectra of the IMFs and residual at different stages are obtained to discriminate the signal sources of the IMFs and residual in the time domain. The power spectral density (PSD) of the impact pressure at different stages was obtained by the FFT:

$$P = \lim_{T \rightarrow \infty} \frac{1}{T} \int_{-T/2}^{T/2} \left(\sum_{i=1}^n \text{IMF}_i(t) + r(t) \right)^2 dt = \frac{1}{2\pi} \int_{-\infty}^{\infty} \lim_{T \rightarrow \infty} \frac{|F_T(\omega)|^2}{T} d\omega \quad (4)$$

$$\text{PSD} = \lim_{T \rightarrow \infty} \frac{|F_T(\omega)|^2}{2\pi T} \quad (5)$$

where $\omega = 2\pi f$ is the angular frequency and f is the frequency. For the released flows in our experiments, eight IMF components are sufficient to describe the impact pressure signals. This is because the main frequency of IMF₈ is lower than 0.5 Hz, which is 0.025% of the sampling frequency.

2.5. Dimensionless Characterization of Flow Regimes

The flow regimes for debris flows are characterized according to the stresses that govern their motion (Iverson, 1997; Savage & Hutter, 1989). The Bagnold number $N_{\text{Bag}} = C_v \rho_g \gamma d_s^2 / (1 - C_v) \eta$ defines the relative dominance between collisional and viscous forces, where γ is the flow shear rate ($\gamma = v/h$). The Savage number $N_{\text{Sav}} = \rho_g d_s^2 \gamma^2 / (\rho_g - \rho_s) g h \tan \varphi$ is the ratio between collisional and frictional forces where φ is the internal friction angle and g is the acceleration due to gravity. The friction number $N_{\text{Fri}} = C_v (\rho_g - \rho_s) g h \tan \varphi / (1 - C_v) \gamma \eta$ is the ratio between frictional and viscous forces. The grain Reynolds number $N_{\text{Rg}} = \rho_s \gamma d_s^2 / \eta$ is the ratio between the solid inertial stress and the fluid viscous shearing stress.

These dimensionless numbers are typically used to classify the dominant energy dissipation mechanisms in natural (Iverson, 1997) and experimental (de Haas et al., 2015) debris flows. Collisional forces dominate over viscous forces when $N_{\text{Bag}} > 200$; collisional forces dominate over frictional forces when $N_{\text{Sav}} > 0.1$ (Bagnold, 1954; Savage & Hutter, 1989). When $N_{\text{Fri}} > 250$, frictional forces dominate over viscous forces (de Haas et al., 2015; Parsons et al., 2001). Generally, debris flows begin to show inertial effects and deviate significantly from ideal viscous behavior when $N_{\text{Rg}} > 1$ (Vanoni, 1975).

3. Results and Analysis

In this section, we first describe the general characteristics of the experimental debris flows. Then we discuss the flow regimes of the debris flows with various volume concentrations in terms of their dimensionless numbers. Next, we identify the components of impact pressure signals. Finally, we assess the effects of flow regime and volume concentration on the impact pressures.

3.1. General Flow Characteristics

Following opening of the headgate, a flow quickly initiated as a result of the dam-break initial condition and strong longitudinal thrust imparted by subsequent flow. The released flows with slurry density $\rho_s = 1,000\text{--}1,300 \text{ kg/m}^3$ flowed turbulently downwards. By contrast, for released flows with $\rho_s = 1,350 \text{ kg/m}^3$ flow behavior was similar to a plug flow in the depth direction due to the high viscosity (Figure S3 in Supporting Information S1). The coarse grains (5–8 mm), shown in brown on Figure 5, segregated upwards to the surface of a debris flow with a low slurry density and were then preferentially transported to the front by the bulk flow, where they could be overrun, recirculated, and accumulated. However, no segregation of coarse grains was observed for debris flows with $\rho_s = 1,350 \text{ kg/m}^3$. Each agitated flow impacted the steel plate mounted to the pressure sensor and was fully diverted upwards (Figure S4 in Supporting Information S1), producing a jet-like flow at the flume exit due to a high Froude number representing the relative effects between flow inertia and gravity.

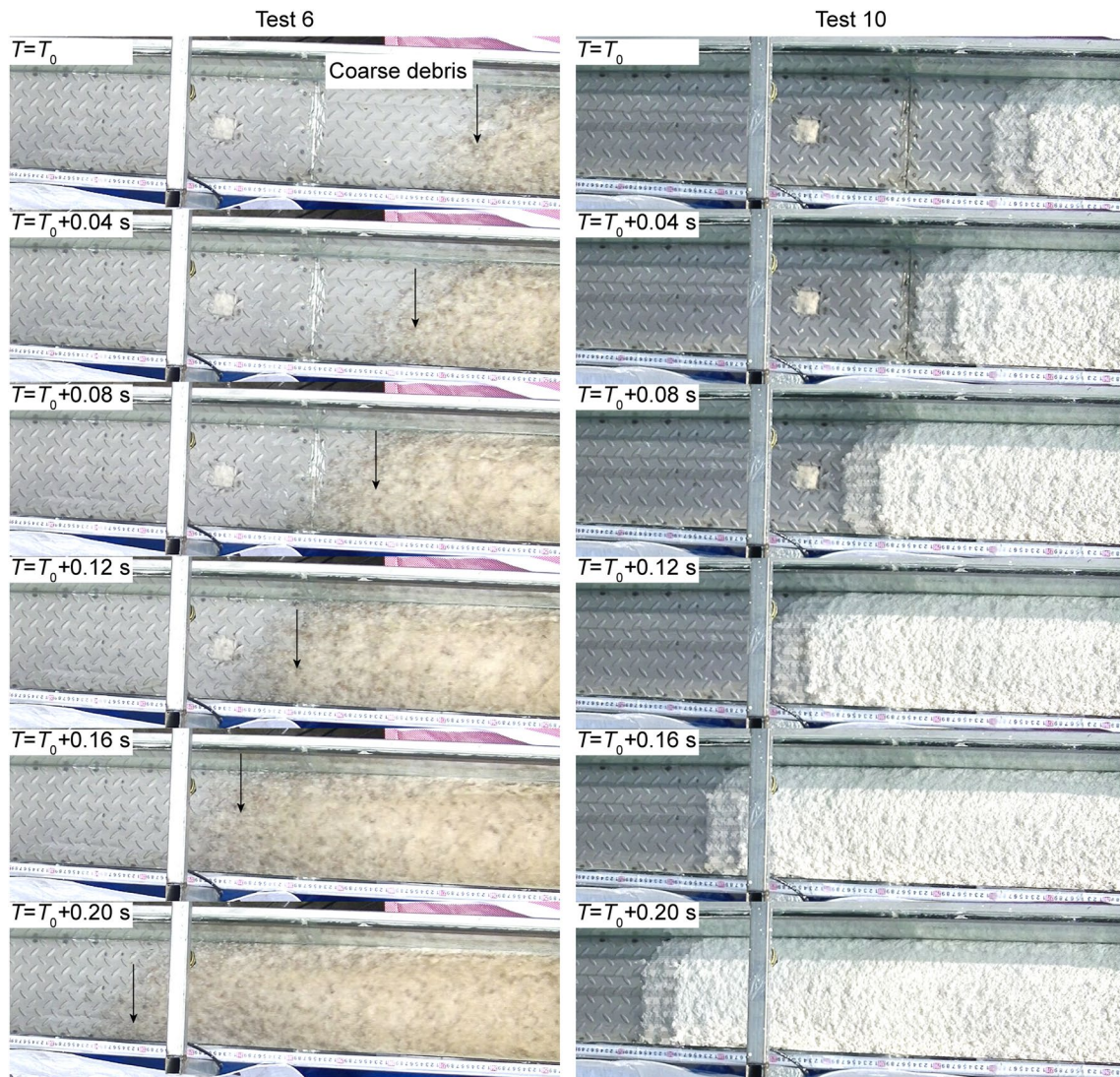


Figure 5. Top view of the propagation process of debris flows in tests 6 and 10. Significant segregation of coarse grains (5–8 mm) in brown, indicated by black arrows, was observed in test 6 but not in test 10. T_0 is the time corresponding to the first frame.

The flow-front velocity at the flume exit in the experimental runs was between 2.85 and 4.90 m/s (Figure 6 and Table S1 in Supporting Information S1). The flow-front velocity first increased with increasing slurry density but decreased when ρ_s exceeded 1,200 kg/m³. The flow-front velocity decreased with increasing debris-grain concentration. The flow-front depth at the flume exit was in the range 0.05–0.07 m. It generally decreased with increasing slurry density and did not show a significant dependence on the debris-grain concentration.

3.2. Flow Regimes

Debris flows transition from collisional to viscous flow regimes and from frictional to viscous flow regimes with increasing slurry density (Figures 7a–7c). By contrast, the effect of debris-grain concentration on the flow regime is minor. Collisional forces dominate over viscous forces when $\rho_s = 1,000$ –1,100 kg/m³ and a reverse tendency occurs when $\rho_s = 1,100$ –1,350 kg/m³ (Figure 7a). Collisional forces were dominated by frictional forces for each debris flow presented in Figure 7b. Frictional forces dominate over viscous forces when $\rho_s = 1,000$ –1,200 kg/m³ while flows present primarily viscous behavior when $\rho_s = 1,300$ and 1,350 kg/m³ (Figure 7c). Grain interactions become more effectively buffered as the slurry viscosity in the pores increases because the fluid inertia increasingly outweighs the grain inertia (Figure 7d).

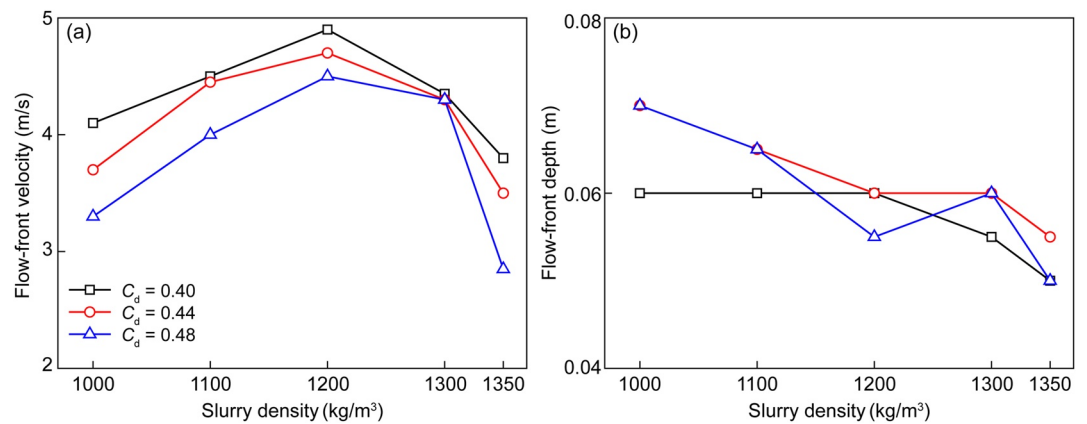


Figure 6. Flow-front velocity (a) and depth (b) of debris flows at the flume exit.

3.3. Features of Impact Pressure Spectra

The impact pressure spectra of released slurries and debris flows at different stages were analyzed based on the comparisons between tests 1 and 6 and between tests 2 and 7 (Figures 8 and 9, respectively). The respective densities of pure slurries in tests 1 and 2 are 1,000 and 1,100 kg/m³, which are identical to those of the debris flows in tests 6 and 7 with $C_d = 0.40$. The decomposition process of impact pressure signals is shown in Figures S5–S6 in Supporting Information S1. The comparisons of impact pressure spectra between pure slurries and debris flows with the same slurry density are similar for other tests.

For clear water in test 1, sampling noise from the data collecting instrument with main frequencies of 50, 150, 250, and 350 Hz was detected before flows were released (Figure 8b). Resonance of the experimental apparatus

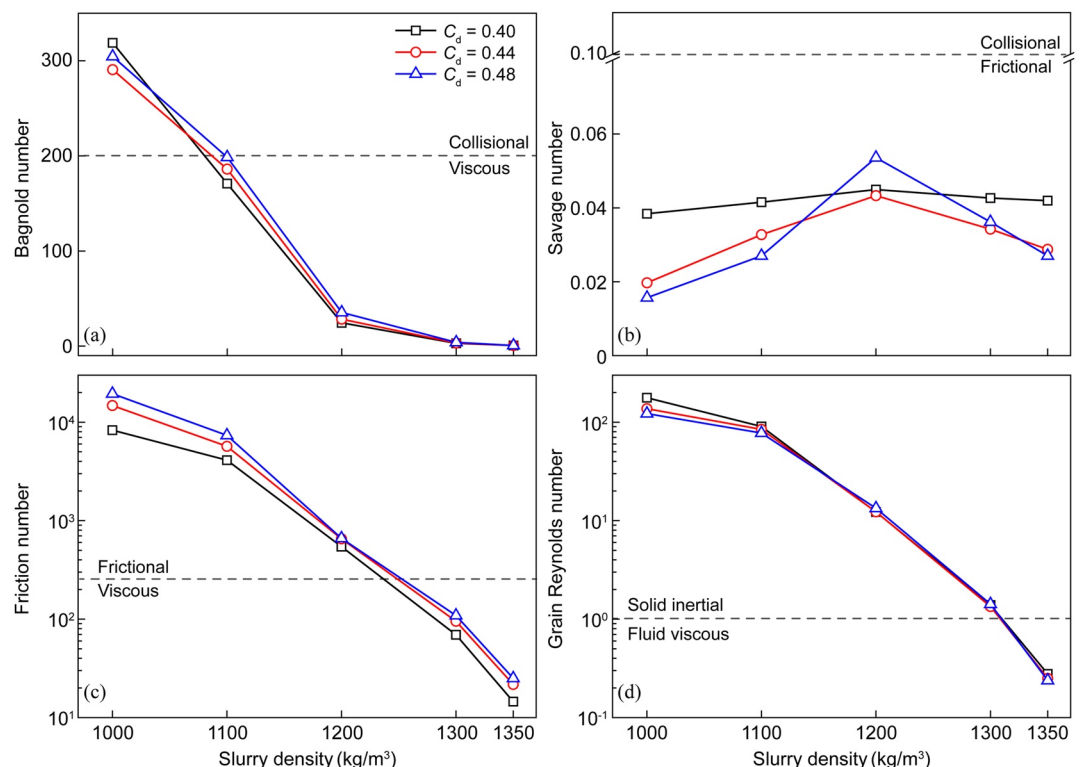


Figure 7. Effects of slurry density and debris-grain concentration on the flow regime: (a) Bagnold number; (b) Savage number; (c) friction number; (d) grain Reynolds number.

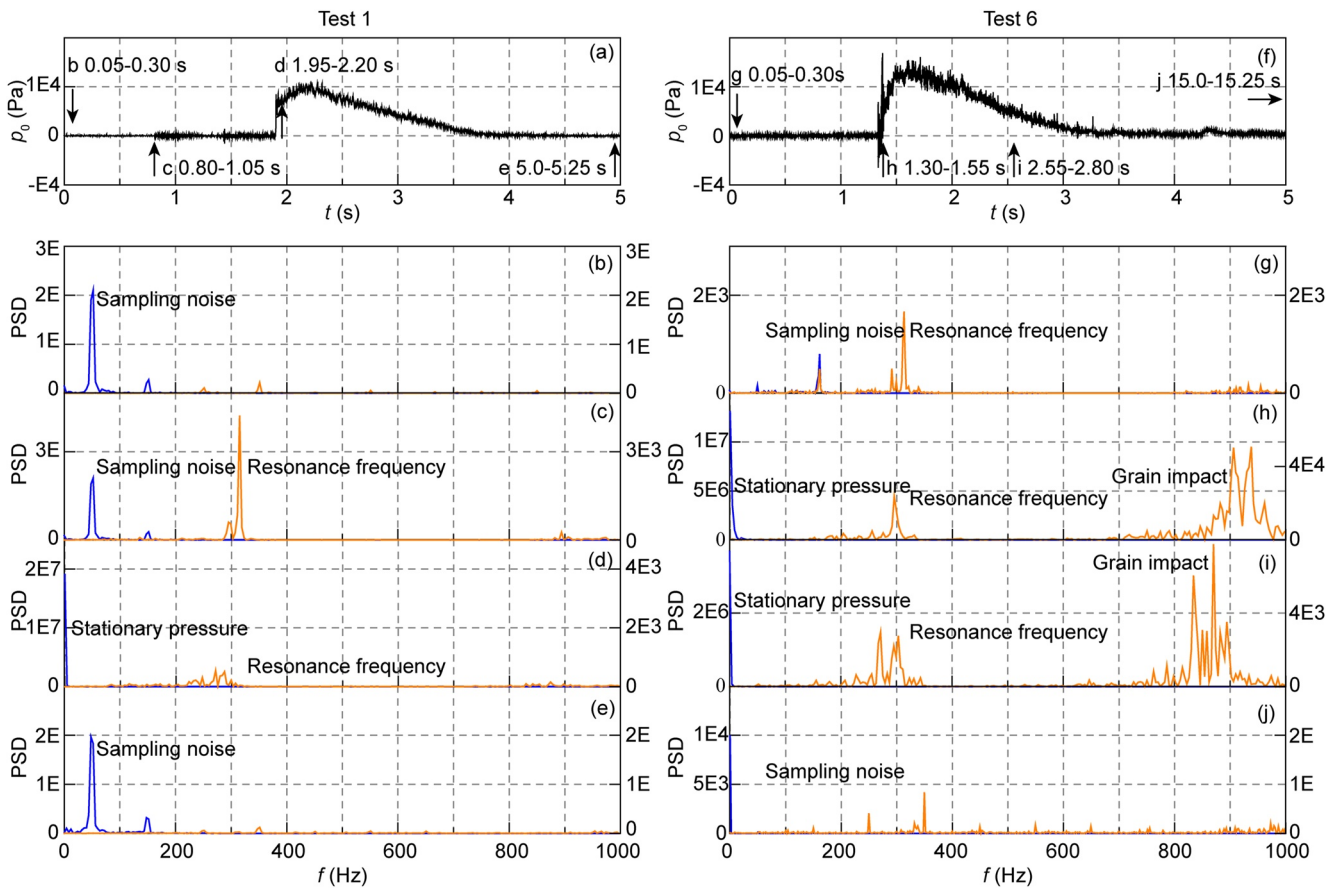


Figure 8. Power spectral densities (PSDs) of impact pressure signals at different stages of tests one and six. The orange lines denote the PSDs of IMF1 and IMF2, while the blue lines denote the PSDs of IMF3–IMF8 and the residual for each test. (b–e) display the PSDs of pure slurry in test 1 at different stages in (a). Similarly, (g–j) display the PSDs of debris flow in test 6 at different stages in (f). The resonance of the experimental apparatus in test 6 prior to flow release was due to the vibration of the blenders.

at a main frequency around 300 Hz occurred when the headgate was opened (Figure 8c). The resonance intensity gradually attenuated as the flow passed through the flume channel. The sampling noise and resonance of the experimental apparatus were eliminated from the measured signal to enable a precise analysis of the impact pressure induced by the released flows. When the bulk flow reached the pressure sensor at the flume exit, a SMP was developed which resulted in a frequency smaller than 20 Hz on the spectra (Figure 8d). After the entire flow went through the flume exit, only sampling noise remained.

Compared with the pure slurries, a significant FP with a frequency 800–1,000 Hz occurred for debris flows including debris grains during the impact process (Figures 8h and 9h). The FP at the flow front is much stronger than that at the flow body (Figures S5 and S6 in Supporting Information S1). The local fluctuations for the debris flow in test 7 appeared when coarse debris grains that had segregated from the bulk flow impacted the sensor prior to bulk flow (Figure S7 in Supporting Information S1). At a time of 0.08 s later, the bulk flow reached the sensor, generating an impact pressure consisting of a SMP and a FP. On this basis, it is inferred that the FP arose from the collision of coarse grains. It is consistent with the physical model proposed by Farin et al. (2019) that a high-frequency seismic signal is generated by the collision of coarse grains. The PSD of the SMP is several orders of magnitude higher than that of the FP due to a long-lasting flow pressure.

The impact pressure p_b of each debris flow was recomposed from the bulk-flow induced SMP (main frequency <20 Hz) and the FP (800–1,000 Hz) from the hard impact of coarse grains without considering the sampling noise, environmental noise and resonance frequency (Figure 10 and S8 in Supporting Information S1). Only small deviations are observed between the measured impact pressure p_0 and recomposed impact pressure p_b , displaying

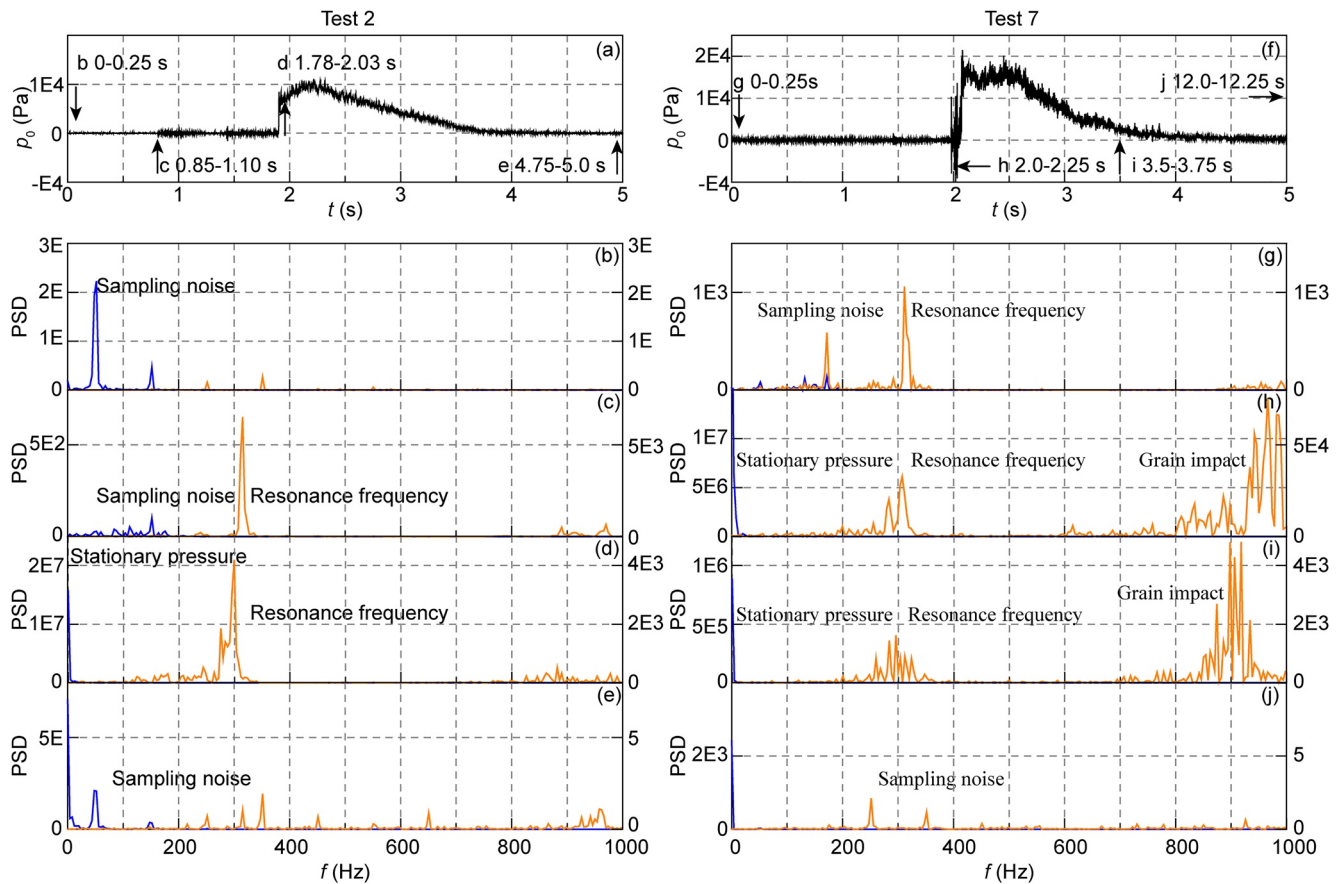


Figure 9. Power spectral densities of impact pressure signals at different stages of tests 2 and 7.

a high signal-to-noise ratio. The peak FPs of all debris flows appeared before the respective peak SMPs were reached. The peak impact pressure p_{bm} coincided with the peak FP (test 6) or with the peak SMP (all other tests).

3.4. Stationary Mean Pressures From the Bulk Flow

Inertia prevails over gravity for each flow during the impact process due to a high Froude number F_r (larger than 4.2, as shown in Table S1 in Supporting Information S1), contributing to the formation of an upward jet. An analytical model (jet model) for flow impact against an obstacle is established using the Bernoulli equation to predict peak SMP p_{mm} (Figure S9 in Supporting Information S1).

$$z_1 + \frac{p_1}{\rho_b g} + \frac{\alpha_1 v_1^2}{2g} = z_2 + \frac{p_2}{\rho_b g} + \frac{\alpha_2 v_2^2}{2g} \quad (6)$$

where z_1 , p_1 , v_1 represent the height above the reference plane, pressure, and mean flow velocity at the incident flow front and z_2 , p_2 , v_2 represent the corresponding values at the top of the jet flow. The energy loss caused by turbulent and viscous stress is not calculated in Equation 6 considering the incident flow front is close to the jet flow. α is the correction coefficient of flow kinetic energy. α is taken as 1.0 for the released slurries and debris flows in our experiments as a result of a large F_r (Song et al., 2021). Assuming a hydrostatic pressure distribution in the depth direction, the mean impact pressure p_w exerted on the steel panel is found to be

$$p_w = p_1 (1 + 0.5\alpha_1 F_r^2) - \rho_b g h_o \quad (7)$$

where h_o is the vertical distance from the panel center to the flume bottom. A detailed derivation is provided in Supporting Information S1.

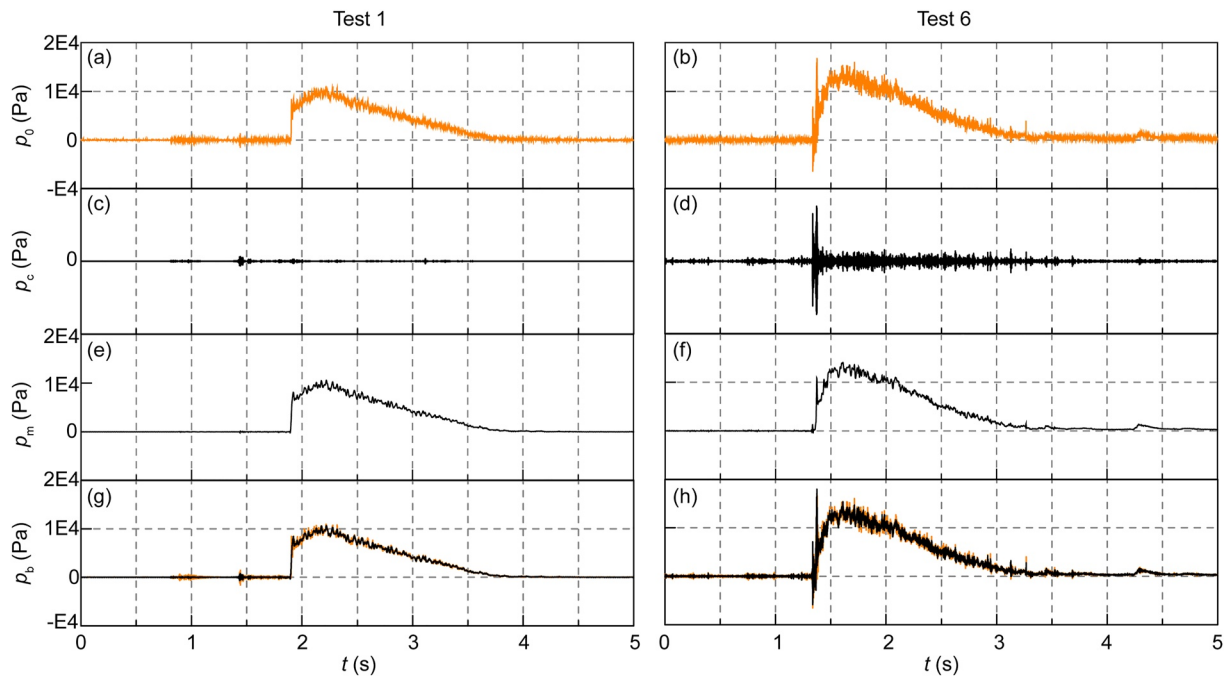


Figure 10. Recomposition of impact pressures with the stationary mean pressure (SMP) and fluctuating pressure in tests 1 and 6: (a, b) impact pressure signal p_0 ; (c, d) FP p_c from the hard impact of coarse grains; (e, f) SMP p_m ; (g, h) recomposed impact pressure p_b .

As shown in Figure 11, the impact pressures p_w calculated by the jet model accurately predict the peak SMP p_{mm} measured from the impact tests of all slurries and debris flows. This indicates that the jet model based on the Bernoulli equation is able to calculate the impact pressure caused by fluid-phase flow thrusting.

We find a clear maximum in the relations between the peak SMP and the slurry density. p_{mm} and p_w first increase with increasing slurry density and then decrease when ρ_s is larger than $1,200 \text{ kg/m}^3$, regardless of debris-grain

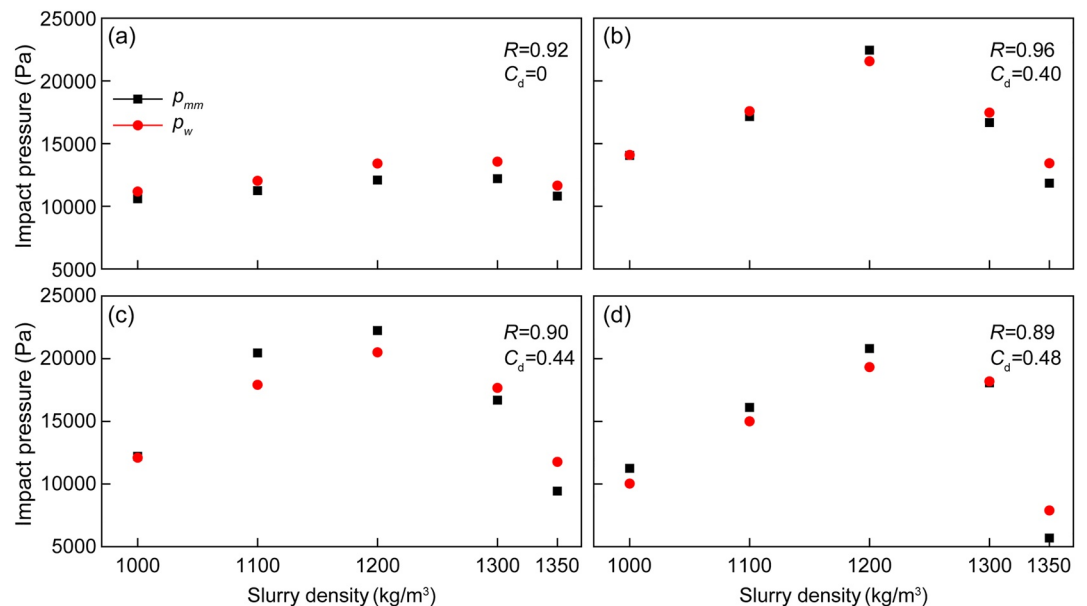


Figure 11. Comparison between the maximum stationary mean pressure measured from impact tests, p_{mm} , and the impact pressure calculated from the analytical model, p_w : (a) pure slurries in tests 1–5; (b) debris flows in tests 6–10; (c) debris flows in tests 11–15; (d) debris flows in tests 16–20.

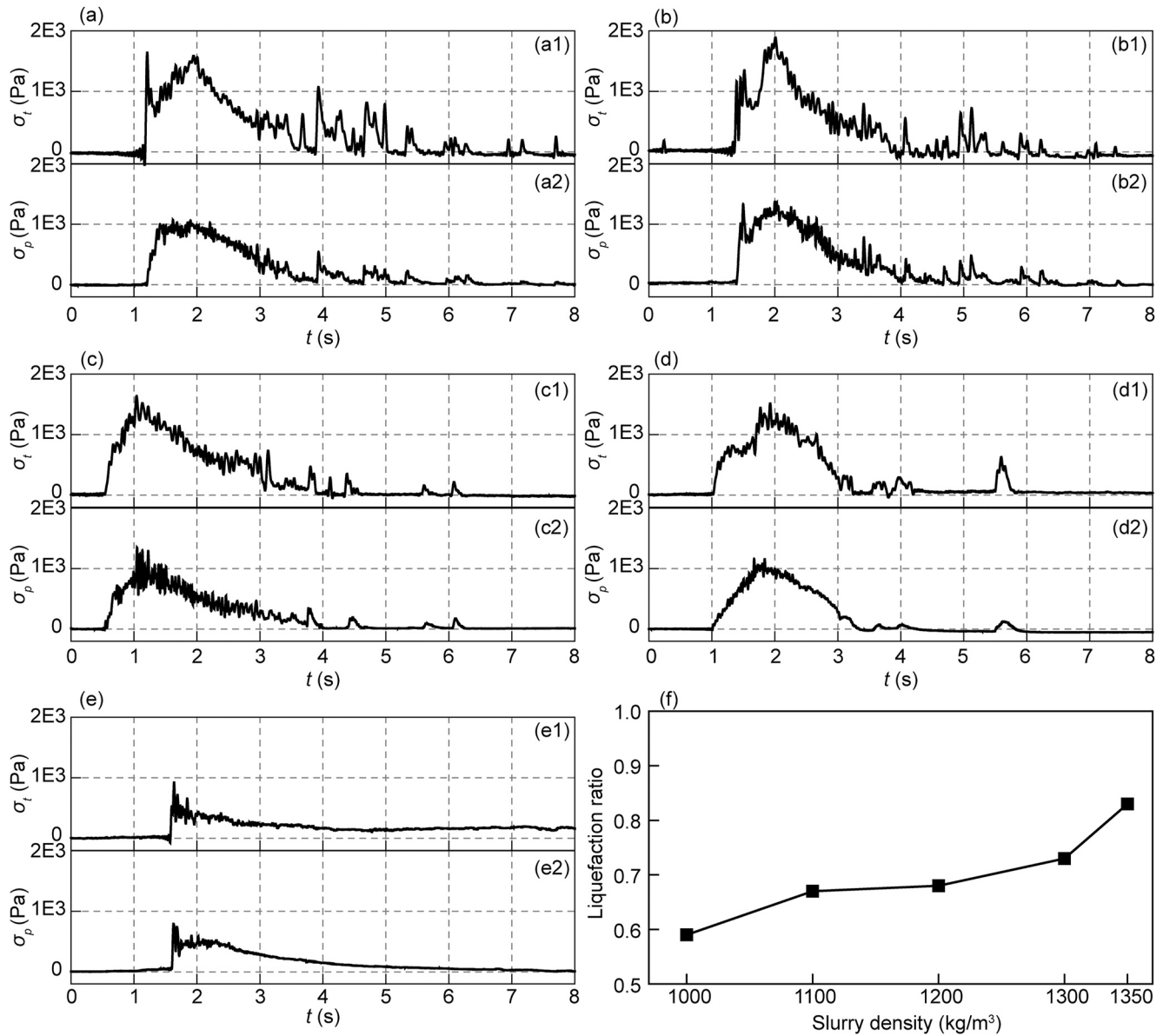


Figure 12. Measurements of basal total normal stress, $\sigma_t(t)$, and basal pore pressure, $\sigma_p(t)$, at $x = 3.2$ m (a–e) and liquefaction ratio (f) in tests 6–10. Filtering is adopted to eliminate fluctuations of $\sigma_t(t)$ and $\sigma_p(t)$. After debris flow passes over the pore-pressure sensor, part of the debris flows in tests 10, 15 and 20 is deposited on the bottom of the flume due to viscous slurry and thus $\sigma_t(t)$ and $\sigma_p(t)$ are greater than 0.

concentration. The bulk density of the released flow increases with increasing slurry density and thus the impact pressure increases with increasing hydrostatic pressure of the incident flow (Equation 7). On the other hand, debris flows with $\rho_s = 1,000\text{--}1,200$ kg/m³ are within the frictional flow regime (Figure 7). The liquefaction ratio of debris flow increases with increasing slurry density (Table S1 in Supporting Information S1). This is because denser slurries have a higher viscosity (Figure 2a) and lower diffusion coefficient of pore-fluid pressure (de Haas et al., 2015). The effective stress and the corresponding basal shear stress of debris flows are significantly reduced by the nonequilibrium pore-fluid pressure caused by the increase of slurry density (Figure 12). The mean flow-front velocity is enhanced because of the low basal shear resistance (Figure 6), resulting in increased impact pressures. By contrast, debris flows transition into the viscous flow regime when $\rho_s > 1,200$ kg/m³. The mean flow-front velocity is reduced by the enhanced viscous resistance (Figure 6) and thus impact pressures of debris flows decrease.

The impact pressures of debris flows generally decrease with the increase of debris-grain concentration except for $\rho_s = 1,300 \text{ kg/m}^3$ (Table S1 in Supporting Information S1). The basal frictional resistance increases with the increase of debris-grain concentration for $\rho_s = 1,000\text{--}1,200 \text{ kg/m}^3$ and significant viscous resistance tends to retard the motion of debris grains for $\rho_s = 1,350 \text{ kg/m}^3$, reducing the mean flow-front velocity and impact pressure. By contrast, liquefaction ratios of debris flows are larger than 0.73 and viscous resistance is relatively limited for debris flows with $\rho_s = 1,300 \text{ kg/m}^3$. The mean flow-front velocity slightly decreases with the debris-grain concentration for $\rho_s = 1,300 \text{ kg/m}^3$ due to the low shear resistance (Figure 6). The increase in impact pressures of debris flows is attributed to the increase of bulk density and hydrostatic pressure of the incident flow (Equation 7).

3.5. Fluctuating Pressure From Collisions of Coarse Grains

Compared with the SMPs, the PSDs of FPs were relatively low due to the short duration of grain collisions. However, the peak FP p_{cm} was more than 20% of p_{mm} for all debris flows presented here (Table S1 in Supporting Information S1). Its significance makes it necessary to take the FP into account when predicting the impact pressure of debris flows.

The FP p_g from the collision of coarse grains can be derived based on the impulse–momentum theorem (e.g., Bugnion et al., 2012; Scheidl et al., 2013):

$$p_g = \beta \rho_c v^2 \quad (8)$$

where the degree of segregation of coarse grains β quantifies the accumulation of coarse grains at the flow front (Zhou et al., 2020). β is calculated from the FP p_g and flow-front velocity v . As shown in Figure 13a, the measured p_{cm} are well represented by the quadratic velocity-dependent formula by assuming a degree of segregation of coarse grains $\beta = 2.0, 1.2,$ and 1.1 for $C_d = 0.40, 0.44,$ and 0.48 . The FPs p_{cm} and p_g increase first with increasing slurry density and then decrease when ρ_s is larger than $1,200 \text{ kg/m}^3$, regardless of debris-grain concentration (Figure 13b). The variation of the FP with slurry density has a consistent trend with that of the SMP.

The back-calculated β based on Equation 8 is in the range 0.8–2.2 in the experimental runs. Particle size segregation occurs in debris flows when small particles preferentially fall down into randomly occurring voids beneath them while large particles move up to the free surface (Vallance & Savage, 2000). This process can be explained by gravity-induced and shear-gradient-induced segregation mechanisms (Hill & Tan, 2014; Weinhart et al., 2013). For the former, the contact stress gradient of coarse grains is higher than that of fine grains. For the latter, coarse grains are segregated to the free surface in the region of low kinetic stress. The gradients of contact stress and kinetic stress can effectively push the coarse grains upwards (Staron & Phillips, 2015).

On this basis, we attempt an interpretation of the tendency of coarse grains to segregate with slurry density (Figure 13b). A simplified model of segregation rate q proposed by May et al. (2010) is expressed as

$$q = s_r \gamma \quad (9)$$

where s_r is a non-dimensional segregation number which is directly proportional to the reduced gravitational acceleration $g' = (\rho_g - \rho_s) g / \rho_g$ due to buoyancy (Gray & Thornton, 2005; Zhou et al., 2020). The flow velocity and shear rate of the debris flows presented here increase with slurry density when $\rho_s < 1,200 \text{ kg/m}^3$. The kinetic stress gradient is enhanced by the increased flow shear rate (Staron & Phillips, 2015). On the contrary, the contact stress gradient of coarse grains is reduced due to the increase in the buoyancy of ambient slurry (Zhou et al., 2020). Accordingly, the degree of segregation of coarse grains initially increases slightly with slurry density due to the dual control of slurry buoyancy and shear rate (Equation 9). By contrast, a rapid decrease of the degree of segregation of coarse grains occurs for $\rho_s > 1,200 \text{ kg/m}^3$. Fluid drag stresses become significantly enhanced in viscous flows considering slurry viscosities at $\rho_s = 1,300$ and $1,350 \text{ kg/m}^3$ exceed those at $\rho_s = 1,000$ and $1,100 \text{ kg/m}^3$ by two orders of magnitude (Figure 2a). The contact stress gradient in viscous flow is significantly counteracted by the buoyancy and fluid drag stresses of ambient slurry with high density. Moreover, viscous stresses induce the formation of nearly plug flows in tests 10, 15, and 20 (Figure S3 in Supporting Information S1) wherein local shear rates are reduced; the segregation of coarse grains is thus significantly inhibited.

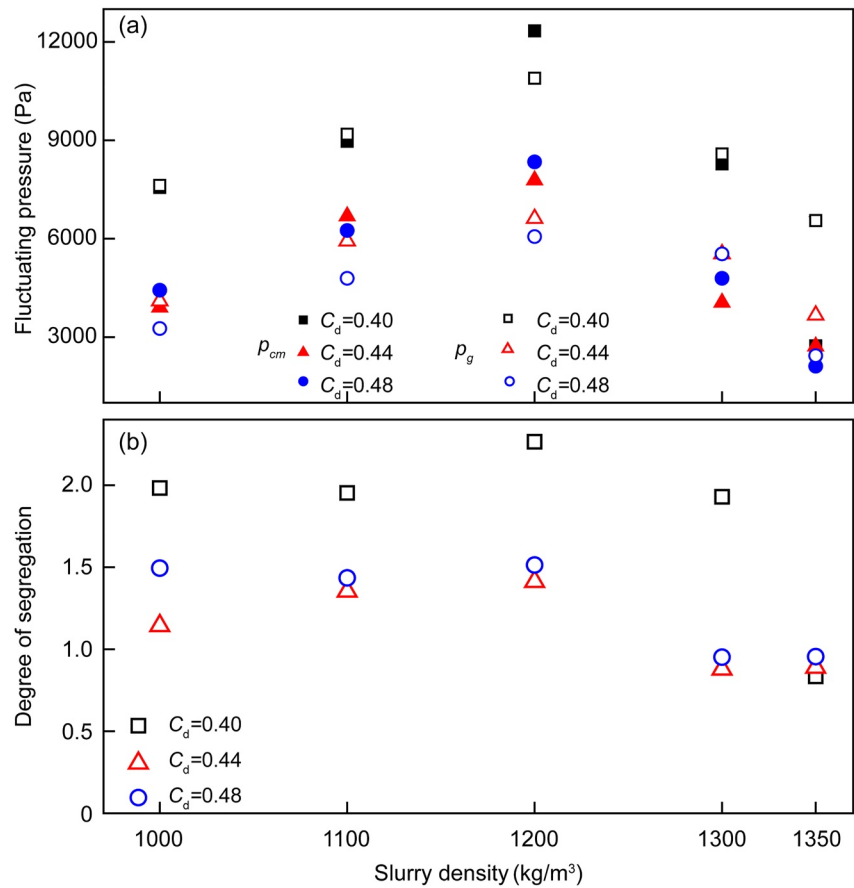


Figure 13. (a) Comparison between the maximum fluctuating pressure measured from impact tests, p_{cm} , and the collision pressure calculated from the impulse–momentum theorem, p_g , assuming a degree of segregation of coarse grains $\beta = 2.0, 1.2,$ and 1.1 for $C_d = 0.40, 0.44,$ and 0.48 . (b) The back-calculated β based on Equation 8 in each test.

4. Discussion

4.1. Characteristics of the Fluctuating Pressure

Peak FPs appear before peak SMPs due to the fast transport of coarse grains at the flow front (Figures S5 and S6 in Supporting Information S1). Subsequently, the FP rapidly attenuates because a significant local dynamic fluid pressure develops near the steel panel during the impact process (Figure 14). The resulting dynamic fluid-pressure gradient tends to retard the impact of the debris grains on the steel panel due to the increased fluid drag forces on the solid phase. This interacting process is revealed by Levy and Sayed (2008) using a two-phase flow model. Finally, the measured impact pressure of debris flow gradually declines to the dynamic fluid pressure at the rear of the debris flow with finer-grained, more dilute and liquefied materials.

The FPs in our experiments are analogous to seismic vibrations induced by debris flows where significant ground velocities and normal-stress fluctuations are caused by coarse-grain collisions (de Haas et al., 2021; Farin et al., 2019). Strong normal-stress fluctuations with a high frequency occur at the flow front and rapidly decrease at the flow body as a result of grain segregation (de Haas et al., 2021). In addition, the seismic vibrations of debris flows are also enhanced by the concentration of coarse grains and suppressed by the slurry density.

Flow velocity has a vital influence on the FP and SMP of a debris flow indicated by Equations 7 and 8. The effect of the depth of the incident flow on the FP and SMP is secondary. However, runup of debris flows against obstacles is closely related to the depth of incident flow (Iverson et al., 2016). The peak FP for a natural debris flow can be enhanced when flow-front velocity is higher than the measured values in this study. Hence, it is of importance to take the FP into account when predicting the impact pressure of debris flows.

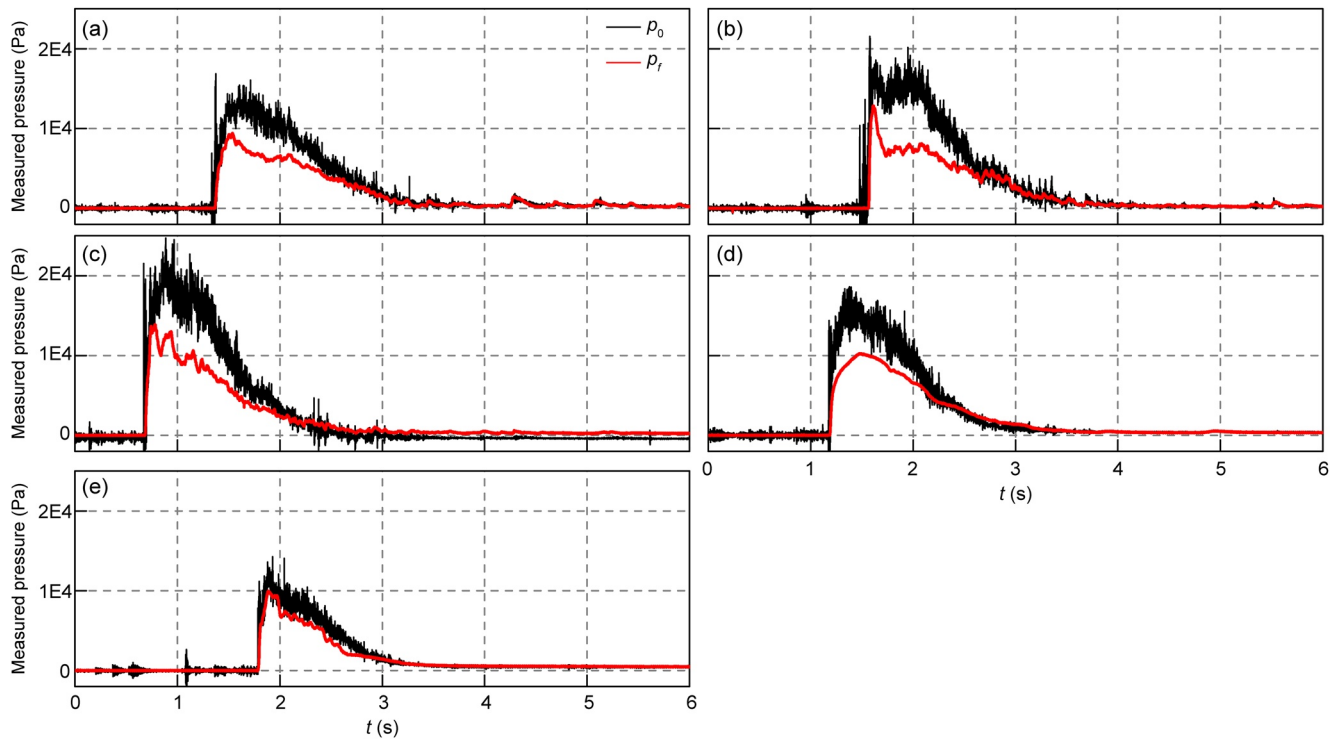


Figure 14. Measured impact pressure, $p_0(t)$, and dynamic fluid pressure, $p_f(t)$, at the flume exit (a–e) in tests 6–10.

4.2. Impact Coefficient of the Hydrodynamic Formula

The peak impact pressure p_{bm} recomposed from the SMP and the FP can be described by the hydrodynamic formula (Equation 2). The calculated λ is in the range 0.4–0.7 in our experiments (Figure 15) and consistent with reported values (0.4–0.8) for hillslope debris flows in Veltheim (Bugnion et al., 2012) and the values (0.3–0.7) for the debris flows in British Columbia (Hung et al., 1984) and Jiangjia Ravine (Hu et al., 2011).

The impact coefficient generally decreases with the increase of slurry density for all released flows. The impact coefficients of pure slurries are smaller than those of debris flows with the same slurry density and the difference between these two sets of impact coefficients gradually narrows with the increase of slurry density (Figure 15). The effect of intergranular stresses including frictional and collisional stresses relative to viscous stresses becomes weakened with increasing slurry density due to increases in the viscosity of slurry and liquefaction ratio of debris flow (Kaitna et al., 2016). The impact pressure produced by the collision of debris grains is suppressed by the ambient slurry, resulting in a decrease in the impact coefficient. The measured dynamic fluid pressures approximate the corresponding impact pressures in tests 10, 15, and 20 (Figure 14), indicating that debris flows display prominent fluid viscous rather than grain inertia (Figure 7d).

The impact coefficients measured from debris flows in some model experiments and in the field can be larger than unity (P. Cui et al., 2015; Scheidl et al., 2013; Watanabe & Ikeya, 1981). The reason for the difference is because the sizes of coarse grains in the debris flow in those measurements are similar in magnitude to the panel which bears the impact pressure. A point-wise loading is induced by each coarse grain collision (Scheidl et al., 2013), causing a stress concentration on the panel. The magnitude of point impact depends on the kinetic energy, diameter of coarse grains and contact deformation which can be estimated using the Hertz model (Hung et al., 1984). The amplitude

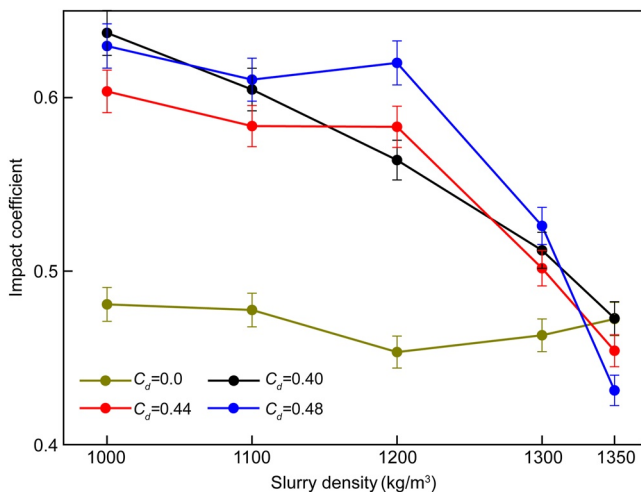


Figure 15. Impact coefficient λ for the experimental debris flows presented here. The error bars for λ are inferred from the errors in flow-front velocity.

of point-wise impact significantly decreases as the panel size is increased (Bugnion et al., 2012; Iverson, 1997). The effect of point-wise loading on the impact pressure is negligible in our experiments because the panel size is more than 10 times the diameter of the coarse grains.

4.3. Segregation of Coarse Grains

The degree of segregation of coarse grains in all tests is less than 2.2 because of a short migration distance (4.0 m). For a natural debris flow, the segregation of coarse grains can be more pronounced due to migration distances of several kilometers or even tens of kilometers (de Haas et al., 2018; Iverson, 1997).

Our experiments show that the buoyancy of the ambient slurry has a negative effect on the grain segregation in debris flows. These results are consistent with experiments conducted using chute flows (de Haas et al., 2015; Vallance & Savage, 2000; Zanuttigh & Ghilardi, 2010) and simulations of grain segregation in flows with different interstitial fluid (Pudasaini & Fischer, 2020; Zhou et al., 2020). This is because the contact stress gradient of debris grains is counteracted by the buoyancy of ambient slurry. The grain segregation is sensitive to the shear rate of debris flow, which is in accord with numerical observations (Itoh & Hatano, 2019; Staron & Phillips, 2015).

More importantly, our results show that grain segregation diminishes as the fluid viscosity increases but intensifies when the viscosity is below a certain threshold value (i.e., 0.015 Pa s). The transition in the dependence of grain segregation on viscosity coincides with the transition of the flow rheology from being dominated by viscous or frictional stresses (Figure 7). This transition is consistent with the numerical simulation of grain segregation using computational fluid dynamics coupled with the discrete element method (K. F. E. Cui et al., 2021). In viscous flows, fluid drag stresses become relevant which weaken contact stress gradients necessary in driving coarse grains upward. In the frictional regime, the flow velocity and shear rate are enhanced by the nonequilibrium pore-fluid pressure (Figures 6 and 12), which in turn boost kinetic stress gradients and contribute to the segregation of coarse grains.

4.4. Implications

We have proposed a method to predict the SMP and the FP of debris flows based on their physical properties. The predictive models of impact pressure are applicable to experimental debris flows with collisional, viscous or frictional flow regimes (Figure 7). This means that the predictive models are appropriate for natural debris flows with different regimes. Gravel with a diameter of 5–8 mm is the segregated debris at the flow front in this study. However, the diameter for grain segregation in natural debris flows with particles ranging from clay to large boulders is still an open question (Cui et al., 2021). The impact pressures of debris flows exerted on structures can be estimated according to a certain weight coefficient for the combination of the SMP and the FP, considering that these two pressures do not peak simultaneously (Figure 10).

The predictive models of impact pressure can be used for the designs of infrastructure and disaster-mitigation structures in mountainous regions. In addition, these models may be suitable for other geophysical flows composed of solid and fluid phases, like pyroclastic flows which are ground-hugging, dense, gas–particle mixtures generated during volcanic eruptions (Roche et al., 2013).

5. Conclusions

We experimentally investigate the impact pressure characteristics of debris flows. In particular, the measured impact pressure signal is decomposed into a SMP and local fluctuations from different sources through EMD. The main concluding remarks are:

1. The impact pressure of each debris flow is decomposed into the SMP with low frequency from the bulk flow and the FP with high frequency from the coarse debris grains. The peak stationary mean pressures are predicted well with the Bernoulli equation and the peak FPs are efficiently described by the impulse–momentum theorem
2. The peak stationary mean pressures and FPs first increase with increasing slurry density and then decrease. This is due to the dual control exerted by the basal frictional stress and viscous stress of debris flows

3. The transition in the dependence of grain segregation on viscosity coincides with the transition of the flow rheology from being dominated by viscous or frictional stresses. The impact coefficient of debris flows generally decreases with the increase of slurry density due to the local dynamic fluid pressure

Notation

| | |
|------------|---|
| C_d | volume concentration of debris grains |
| C_v | volume concentration of a debris flow |
| d_s | mean particle diameter |
| f | frequency |
| F_r | Froude number |
| g | gravitational acceleration |
| h | flow-front height |
| h_0 | vertical distance from the panel center to the flume bottom |
| i | hydraulic gradient |
| IMF_i | the i th intrinsic mode function |
| k | empirical factor for hydrostatic model |
| N_{Bag} | Bagnold number of debris flow |
| N_{Fri} | friction number of debris flow |
| N_{Rg} | grain Reynolds number of debris flow |
| N_{Sav} | Savage number of debris flow |
| p_0 | measured impact pressure of a flow |
| p_1 | pressure at the incident flow front |
| p_2 | pressure at the top of the jet flow |
| p_b | recomposed impact pressure of a flow |
| p_c | fluctuating pressure for coarse grains |
| p_{bm} | peak impact pressure of a flow without noise |
| p_{cm} | peak fluctuating pressure |
| p_{mm} | peak stationary mean pressure of a flow |
| p_{peak} | peak debris-flow impact pressure |
| p_w | impact pressure exerted on the steel panel |
| q | segregation rate of coarse grains |
| r | residual |
| R^2 | determination coefficient |
| s_r | non-dimensional segregation number |
| v | flow-front velocity |
| v_1 | flow velocity at the incident flow front |
| v_2 | flow velocity at the top of the jet flow |
| V_0 | volume of released flow |
| z_1 | height above the reference plane at the incident flow front |
| z_2 | height above the reference plane at the top of the jet flow |
| α | correction coefficient of flow kinetic energy |
| β | degree of segregation of coarse grains |
| θ | slope angle of the flume |
| ρ_b | bulk density of a debris flow |
| ρ_c | dry density of coarse grains |
| ρ_d | dry density of debris grains |
| ρ_g | solid particle density |
| ρ_s | slurry density |
| ρ_w | water density |
| σ_p | basal pore pressure |
| σ_t | basal total normal stress |
| η | dynamic viscosity of a slurry |

| | |
|-----------|--|
| γ | flow shear rate |
| λ | impact coefficient of released flow |
| ω | angular frequency |
| φ | internal friction angle of debris grains |

Conflict of Interest

The authors declare no conflicts of interest relevant to this study.

Data Availability Statement

For each test, the measured impact pressure signal, dynamic fluid pressure at the flume exit, basal total normal stress and pore pressure at $x = 3.2$ m are available at <https://doi.org/10.5281/zenodo.5148243>.

Acknowledgments

Constructive reviews by the editor Amy East and the associate editor Adam M. Booth and two anonymous reviewers helped to improve the manuscript and are gratefully acknowledged. We acknowledge funding from the Natural Science Foundation of China (No. 42007252). We thank Christian Scheidl and Kaiheng Hu for their discussion on the fluctuating pressure of debris flows.

References

- Bagnold, R. A. (1954). Experiments on a gravity-free dispersion of large solid spheres in a Newtonian fluid under shear. *Proceedings of the Royal Society A*, 225(1160), 49–63.
- Bugnion, L., McArdell, B. W., Bartelt, P., & Wendeler, C. (2012). Measurements of hillslope debris flow impact pressure on obstacles. *Landslides*, 9(2), 179–187. <https://doi.org/10.1007/s10346-011-0294-4>
- Costa, J. E. (1988). Rheologic, geomorphic, and sedimentologic differentiation of water flood, hyperconcentrated flows, and debris flows. In *Flood geomorphology, chapter, rheologic, geomorphic, and sedimentologic differentiation of water floods, hyperconcentrated flows, and debris flows* (pp. 113–122). John Wiley.
- Cui, K. F. E., Zhou, G. G. D., & Jing, L. (2021). Viscous effects on the particle size segregation in geophysical mass flows: Insights from immersed granular shear flow simulations. *Journal of Geophysical Research: Solid Earth*, 126, e2021JB022274. <https://doi.org/10.1029/2021jb022274>
- Cui, P., Zeng, C., & Lei, Y. (2015). Experimental analysis on the impact force of viscous debris flow. *Earth Surface Processes and Landforms*, 40(12), 1644–1655. <https://doi.org/10.1002/esp.3744>
- de Haas, T., Åberg, A. S., Walter, F., & Zhang, Z. (2021). Deciphering seismic and normal-force fluctuation signatures of debris flows: An experimental assessment of effects of flow composition and dynamics. *Earth Surface Processes and Landforms*, 1–14. <https://doi.org/10.1002/esp.5168>
- de Haas, T., Braat, L., Leuven, J. R. F. W., Lokhorst, I. R., & Kleinhans, M. G. (2015). Effects of debris flow composition on runout, depositional mechanisms, and deposit morphology in laboratory experiments. *Journal of Geophysical Research: Earth Surface*, 120, 1949–1972. <https://doi.org/10.1002/2015jf003525>
- de Haas, T., Densmore, A. L., Stoffel, M., Suwa, H., Imaizumi, F., Ballesteros-Cánovas, J. A., & Waskiewicz, T. (2018). Avulsions and the spatio-temporal evolution of debris-flow fans. *Earth-Science Reviews*, 177, 53–75. <https://doi.org/10.1016/j.earscirev.2017.11.007>
- Farin, M., Tsai, V. C., Lamb, M. P., & Allstadt, K. E. (2019). A physical model of the high-frequency seismic signal generated by debris flows. *Earth Surface Processes and Landforms*, 44(13), 2529–2543. <https://doi.org/10.1002/esp.4677>
- Gray, J. M. N. T., & Kokelaar, P. (2010). Large particle segregation, transport and accumulation in granular free-surface flows. *Journal of Fluid Mechanics*, 652, 105–137. <https://doi.org/10.1017/s002211201000011x>
- Gray, J. M. N. T., & Thornton, A. R. (2005). A theory for particle size segregation in shallow granular free-surface flows. *Proceedings of the Royal Society A*, 461, 1447–1473. <https://doi.org/10.1098/rspa.2004.1420>
- Hill, K. M., & Tan, D. S. (2014). Segregation in dense sheared flows: Gravity, temperature gradients, and stress partitioning. *Journal of Fluid Mechanics*, 756, 54–88. <https://doi.org/10.1017/jfm.2014.271>
- Hsu, L., Dietrich, W. E., & Sklar, L. S. (2014). Mean and fluctuating basal forces generated by granular flows: Laboratory observations in a large vertically rotating drum. *Journal of Geophysical Research: Earth Surface*, 119(6), 1283–1309. <https://doi.org/10.1002/2013jf003078>
- Hu, K., Wei, F., & Li, Y. (2011). Real-time measurement and preliminary analysis of debris-flow impact force at Jiangjia ravine, China. *Earth Surface Processes and Landforms*, 36, 1268–1278. <https://doi.org/10.1002/esp.2155>
- Huang, N. E., Shen, Z., Long, S. R., Wu, M. C., Shih, H. H., Zheng, Q., et al. (1998). The empirical mode decomposition and the Hilbert spectrum for nonlinear and non-stationary time series analysis. *Proceedings of the Royal Society A*, 454, 903–995. <https://doi.org/10.1098/rspa.1998.0193>
- Hubl, J., Suda, J., Proske, D., Kaitna, R., & Scheidl, C. (2009). Debris flow impact estimation. In *Eleventh international symposium on water management and hydraulic Engineering* (Vol. 1, pp. 137–148).
- Hungr, O., Morgan, G. C., & Kellerhals, R. (1984). Quantitative analysis of debris torrent hazard for design of remedial measures. *Canadian Geotechnical Journal*, 21, 663–677. <https://doi.org/10.1139/t84-073>
- Itoh, R., & Hatano, T. (2019). Geological implication of grain-size segregation in dense granular matter. *Philosophical Transactions of the Royal Society A: Mathematical, Physical and Engineering Sciences*, 377. <https://doi.org/10.1098/rsta.2017.0390>
- Iverson, R. M. (1997). The physics of debris flows. *Review of Geophysics*, 35(3), 245–296. <https://doi.org/10.1029/97rg00426>
- Iverson, R. M. (2003). *Debris Flow Mechanics and Mitigation Conference*, chap. The debris-flow rheology myth. Mills Press.
- Iverson, R. M., George, D. L., & Logan, M. (2016). Debris flow runup on vertical barriers and adverse slopes. *Journal of Geophysical Research: Earth Surface*, 121, 2333–2357. <https://doi.org/10.1002/2016jf003933>
- Iverson, R. M., Reid, M. E., Logan, M., LaHusen, R. G., Godt, J. W., & Griswold, J. P. (2011). Positive feedback and momentum growth during debris-flow entrainment of wet bed sediment. *Nature Geosciences*, 4, 116–121.
- Johnson, C. G., Kokelaar, B. P., Iverson, R. M., Logan, M., Lahusen, R. G., & Gray, J. M. N. T. (2012). Grain-size segregation and levee formation in geophysical mass flows. *Journal of Geophysical Research*, 117, F01032. <https://doi.org/10.1029/2011jg002185>
- Kaitna, R., Palucis, M. C., Yohannes, B., Hill, K. M., & Dietrich, W. E. (2016). Effects of coarse grain size distribution and fine particle content on pore fluid pressure and shear behavior in experimental debris flows. *Journal of Geophysical Research: Earth Surface*, 121, 415–441. <https://doi.org/10.1002/2015jf003725>

- Levy, A., & Sayed, M. (2008). Numerical simulations of the flow of dilute granular materials around obstacles. *Powder Technology*, *181*(2), 137–148. <https://doi.org/10.1016/j.powtec.2006.12.005>
- Maheshwari, S., & Kumar, A. (2014). Empirical mode decomposition: Theory & applications. *International Journal of Electronic and Electrical Engineering*, *7*(8), 873–878.
- May, L. B. H., Golick, L. A., Phillips, K. C., Shearer, M., & Daniels, K. E. (2010). Shear-driven size segregation of granular materials: Modeling and experiment. *Physical Review E - Statistical Physics, Plasmas, Fluids, and Related Interdisciplinary Topics*, *81*(5), 051301. <https://doi.org/10.1103/physreve.81.051301>
- McCoy, S. W., Kean, J. W., Coe, J. A., Staley, D. M., Wasklewicz, T. A., & Tucker, G. E. (2010). Evolution of a natural debris flow: In situ measurements of flow dynamics, video imagery, and terrestrial laser scanning. *Geology*, *38*, 735–738. <https://doi.org/10.1130/g30928.1>
- Parsons, J. D., Whipple, K. X., & Simoni, A. (2001). Experimental study of the grain-flow, fluid-mud transition in debris flows. *Geology*, *109*(4), 427–447. <https://doi.org/10.1086/320798>
- Pierson, T. C. (2005). *Distinguishing between debris flows and floods from field evidence in small watersheds*. U.S. Geological Survey.
- Pudasaini, S. P. (2012). A general two-phase debris flow model. *Journal of Geophysical Research*, *117*, F03010. <https://doi.org/10.1029/2011jg002186>
- Pudasaini, S. P., & Fischer, J. T. (2020). A mechanical model for phase separation in debris flow. *International Journal of Multiphase Flow*, *129*. <https://doi.org/10.1016/j.ijmultiphaseflow.2020.103292>
- Roche, O., Nino, Y., Mangeny, A., Brand, B., Pollock, N., & Valentine, G. A. (2013). Dynamic pore-pressure variations induce substrate erosion by pyroclastic flows. *Geology*, *41*, 1107–1110. <https://doi.org/10.1130/g34668.1>
- Savage, S. B., & Hutter, K. (1989). The motion of a finite mass of granular material down a rough incline. *Journal of Fluid Mechanics*, *199*, 177–215. <https://doi.org/10.1017/s0022112089000340>
- Scheidl, C., Chiari, M., Kaitna, R., Mullegger, M., Krawtschuk, A., Zimmermann, T., & Proske, D. (2013). Analysing debris-flow impact models, based on a small scale modelling approach. *Surveys in Geophysics*, *34*, 121–140. <https://doi.org/10.1007/s10712-012-9199-6>
- Song, D., Zhou, G., Chen, X., Li, J., Wang, A., Peng, P., & Xue, K. (2021). General equations for landslide-debris impact and their application to debris-flow flexible barrier. *Engineering Geology*, *288*, 106154. <https://doi.org/10.1016/j.enggeo.2021.106154>
- Sovilla, B., Schaer, M., Kern, M., & Bartelt, P. (2008). Impact pressures and flow regimes in dense snow avalanches observed at the Vallée de la Sionne test site. *Journal of Geophysical Research*, *113*, F01010. <https://doi.org/10.1029/2006jg000688>
- Staron, L., & Phillips, J. C. (2015). Stress partition and microstructure in size-segregating granular flows. *Physical Review E - Statistical Physics, Plasmas, Fluids, and Related Interdisciplinary Topics*, *92*(2), 022210. <https://doi.org/10.1103/physreve.92.022210>
- Takahashi, T. (2007). *Debris flows: Mechanics, prediction and countermeasures*. Proc. Monogr. Eng. Water Earth Sci. Taylor and Francis.
- Tiranti, D., Bonetto, S., & Mandrone, G. (2008). Quantitative basin characterisation to refine debris-flow triggering criteria and processes: An example from the Italian Western Alps. *Landslides*, *5*(1), 45–57. <https://doi.org/10.1007/s10346-007-0101-4>
- Vallance, J. W., & Savage, S. B. (2000). Particle size segregation in granular flows down chutes. In *IUTAM symposium segregation in granular materials* (Vol. 81, pp. 31–51). Springer. https://doi.org/10.1007/978-94-015-9498-1_3
- Vanoni, V. A. (1975). *Sedimentation engineering*. America Society of Civil Engineers.
- Watanabe, M., & Ikeya, H. (1981). Investigation and analysis of volcanic mudflows on Mt. Sakurajima, Japan. In *Proceedings of Symposium on erosion and sediment transport measurements* (Vol. 133, pp. 245–256). IAHS Publications.
- Weinhart, T., Luding, S., & Thornton, A. R. (2013). From discrete particles to continuum fields in mixtures. In *Proceedings of the 7th international conference on micromechanics of granular media* (Vol. 1542, pp. 1202–1205). AIP Publishing LLC. <https://doi.org/10.1063/1.4812153>
- Zanuttigh, B., & Ghilardi, P. (2010). Segregation process of water-granular mixtures released down a steep chute. *Journal of Hydrology*, *391*(1–2), 175–187. <https://doi.org/10.1016/j.jhydrol.2010.07.016>
- Zheng, H., Shi, Z., Hanley, K. J., Peng, M., Guan, S., Feng, S., & Chen, K. (2021). Deposition characteristics of debris flows in a lateral flume considering upstream entrainment. *Geomorphology*, *394*, 107960. <https://doi.org/10.1016/j.geomorph.2021.107960>
- Zheng, H., Shi, Z., Peng, M., & Yu, S. (2018). Coupled CFD-DEM model for the direct numerical simulation of sediment bed erosion by viscous shear flow. *Engineering Geology*, *245*, 309–321. <https://doi.org/10.1016/j.enggeo.2018.09.003>
- Zheng, H., Shi, Z., Shen, D., Peng, M., Hanley, K. J., Ma, C., & Zhang, L. (2021). Recent advances in stability and failure mechanisms of landslide dams. *Frontiers of Earth Science*, *9*, 659935. <https://doi.org/10.3389/feart.2021.659935>
- Zheng, H., Shi, Z., Yu, S., Fan, X., Hanley, K. J., & Feng, S. (2021). Erosion mechanisms of debris flow on the sediment bed. *Water Resources Research*, *57*(12), WR030707. <https://doi.org/10.1029/2021wr030707>
- Zhou, G. G. D., Cui, K. F. E., Jing, L., Zhao, T., Song, D., & Huang, Y. (2020). Particle size segregation in granular mass flows with different ambient fluids. *Journal of Geophysical Research: Solid Earth*, *125*(10), e2020JB019536. <https://doi.org/10.1029/2020jb019536>